

REPORT FOR THE PROJECT AND SPECIFICATIONS.

CANOPY DELEGATE ENGINEER.

ACCEPTABLE MATERIALS, BUT IN NO CASE LESS THAN 6 INCHES.

OR ROLLER COMPACTOR.

SLAB ON GRADE

SLAB THICKNESS (IN)	<ul> <li>3/4" OR LARGER</li> <li>AGGREGATE</li> <li>SPACING (FT)</li> </ul>
4	12
5	13
6	15
Г	18
8	2Ø
9	23
10	25

CONCRETE AND REINFORCING

C2 CONCRETE WORK SHALL CONFORM TO ACI SPECIFICATIONS FOR STRUCTURAL CONCRETE STRUCTURAL CONCRETE (ACI 318-14).

LOCATION

SLAB ON GRADE: 4" SLAB ON GRADE: 5" ILLED CELLS, PRECA INTELS & BOND BEAM ASTM C476) - SEE NO OTES: USED TO FILL CORES OF CMU. C4 CONCRETE MIX DESIGN SUBMITTALS. OF SUBMITTALS.

C5 NO CALCIUM CHLORIDE SHALL BE USED IN MIX DESIGNS. HAVE W/C RATIO OF 0.60 OR HIGHER.

C8 WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. LAP MINIMUM DISTANCE OF ONE CROSS WIRE SPACING PLUS 2 INCHES.

BAR DIAMETERS IN CONCRETE.

## GENERAL STRUCTURAL NOTES

# FOUNDATION NOTES (cont'd)

- F4 CONTRACTOR IN CONJUCTION WITH GEOTECHNICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY SOILS OR UNSUITABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH WOULD PREVENT ATTAINMENT OF THE DEGIGN SOIL PRESSURE RECOMMNDED BY THE SOILS REPORT.
  - ALL SOIL PREPARATION SHALL CONFORM TO THE RECOMMENDATIONS CONTAINED IN THE SOILS
  - FOUNDATION DESIGN IS BASED ON THE ASSUMPTION OF IDEAL SOIL CONDITIONS AND AN ALLOWABLE SOIL BEARING BEARING PRESSURE OF 2,000 PSF. THE CONTRACTOR SHALL HIRE A GEOTHECNICAL ENGINEERING FIRM TO PERFORM SUBSURFACE SOIL EXPLORATION AND PRODUCE A SOILD REPORT WITH FOUNDATION ND SOIL PREPARATION RECOMMENDATIONS. SOUTHARD ENGINEERING WILL CONFIRM AND VERIFY FOUNDATION SIZES UPON RECEIPT OF SOILS REPORT ND FINAL CANOPY COLUMN LOAD REACTIONS FROM
- F7 ALL VEGETATION, TOPSOILS, ROOTS AND ORGANIC ZONES SHALL BE STRIPPED AND REMOVED FROM THE CONSTRUCTION AREA FOR A DISTANCE OF AT LEAST 5 FEET BEYOND THE EXTERIOR OF BUILDING FOUNDATION LIMITS. THE DEPTH OF STRIPPING SHALL BE THAT REQUIRED TO REMOVE SIGNIFICANT ROOT ZONES, SMALL TREE STUMPS AND OTHER
  - EXCAVATIONS FOR LARGE STUMPS, ABANDONED UTILITIES, UNDERGROUND TANKS, ETC. SHALL BE BACKFILLED IN LAYERS WITH COMPACTION AND TESTING OF EACH LAYER AS DESCRIBED FOR PLACEMENT AND COMPACTION OF FILL MATERIAL. USE LOOSE BACKFILL LAYER THICKNESS APPROPRIATE FOR THE SIZE OF COMPACTOR BEING USED.
  - AFTER THE SITE HAS BEEN CLEARED, THE EXPOSED SOILS AT THE STRIPPED SURFACE WITHIN AND TO A POINT 5 FEET OUTSIDE THE BUILDING CONSTRUCTION AREA SHALL BE COMPACTED WITH OVERLAPPING PASSES WITH A LIGHT TO MEDIUM WEIGHT VIBRATION DRUM ROLLER HAVING A TOTAL OPERATION STATIC WEIGHT OF 4 TO 6 TONS. DENSITIES OF AT LEAST 35 PERCENT OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1551) SHALL BE UNIFORMLY OBTAINED TO A DEPTH OF AT LEAST 12 INCHES BELOW THE COMPACTED SURFACE. REGARDLESS OF THE DEGREE OF COMPACTION ACHIEVED, A MINIMUM OF EIGHT COMPLETE COVERAGES SHALL BE MADE WITHIN THE BUILDING AREA. THE ROLLER COVERAGES SHALL BE DIVIDED EVENLY INTO TWO PERPENDICULAR DIRECTIONS.
- FIO AFTER COMPLETION OF DENSIFICATION OF EXISTING SOILS, STRUCTURAL FILL SHALL THEN BE PLACED IN LIFTS NOT EXCEEDING 12 INCHES IN LOOSE THICKNESS WHEN USING THE ROLLER PREVIOUSLY DESCRIBED. EACH LIFT SHALL BE THOROUGHLY COMPACTED WITH THE VIBRATORY ROLLER UNTIL DENSITIES EQUIVALENT TO AT LEAST 35 PERCENT OF THE MODIFIED PROCTOR MAXIMUM DRY DENGITY ARE UNIFORMLY OBTAINED. STRUCTURAL FILL SHALL CONSIST OF AN INORGANIC, NONPLASTIC, GRANULAR SOIL CONTAINING LESS THAN 10 PERCENT MATERIAL PASSING THE NO. 200 MESH SIEVE, A RELATIVELY CLEAN SAND WITH A UNIFIED SOIL CLASSIFICATION OF SP OR SP-SM.
- FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE BEARING PRESSURE OF 2000 PSF. THE UPPER 12 INCHES OF SANDY BEARING SOILS IN THE FOOTING EXCAVATION BOTTOMS SHALL BE COMPACTED TO DENSITIES EQUIVALENT TO 35 PERCENT OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY. COMPACTION, OR RECOMPACTION OF THE FOOTING EXCAVATION BEARING LEVEL SOILS LOOSENED BY THE EXCAVATION PROCESS, SHALL BE ACHIEVED BY MAKING SEVERAL PASSES WITH A RELATIVELY LIGHTWEIGHT, WALK-BEHIND VIBRATORY SLED
- FI2 UNLESS NOTED, ALL FOOTINGS SHALL BE CENTERED UNDER COLUMNS, PIERS AND WALLS.
- SOGI UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT, COMPACT INTERIOR FILL TO 35% OF MODIFIED PROCTOR MAXIMUM DRY DENGITY (AGTM DI551). SOIL COMPACTION SHALL BE FIELD-CONTROLLED BY A REPRESENTATIVE TECHNICIAN OF A QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER. SLAB ON GRADE SHALL BE CAST OVER A VAPOR BARRIER.
- 50G2 MAXIMUM SPACING OF CONTROL JOINTS SHALL BE AS SET IN THE TABLE BELOW, OR AS NOTED ON PLANS. THE MORE STRINGENT SHALL APPLY. PATTERNS SHALL BE APPROXIMATELY SQUARE WITH A RATIO OF LONG SIDE TO SHORT SIDE NOT EXCEEDING 1.5 TO 1.
  - GER

MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE CUT SLAB WITHIN 12 HOURS

- SOG3 GENERAL CONTRACTOR SHALL COORDINATE EXACT LOCATION OF SAW JOINTS AND CJ'S WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH.
- CI A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
  - FOR BUILDINGS ACI 301 (LATEST EDITION) AND BUILDING CODE REQUIREMENTS FOR
- C3 ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES.

	28 DAY STRENGTH	SLUMP	COURSE AGGREGATE		
			Min.	MAX.	
	4000 PSI	4" +/-  "	3/4"	1 1/2"	
гніск	4000 PSI	4" +/- 1"	3/4"	1 1/2 "	
I THICKER	4000 PSI (MINIMUM FLEXURAL STRENGTH = 550 PSI.)	4" +/- 1" SEE NOTE 1	3/4"	1 1/2"	
AST M GROUT DTE 2	3 <i>000</i> PSI	8" TO II"	SAND	3/8"	

SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4". SEE MASONRY NOTE MIØ FOR TESTING REQUIREMENTS OF GROUT TO BE

#### EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB-ON-GRADE, COLUMNS, ETC). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION

- PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD I OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPERATE PROPOSED MIX.
- 3. SUBMIT CONCRETE MIX DEGIGN FOR EACH PROPOSED CLASS OF CONCRETE.
- C6 MAXIMUM W/C RATIO OF 0.55 FOR FOOTINGS AND 0.50 FOR OTHER CONCRETE. CMU GROUT SHALL
- C1 REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.
- C9 SPLICE REINFORCING ONLY WHERE SHOWN ON THE DRAWINGS. WHERE CONTINUOUS REINFORCING IS CALLED OUT, SUCH REINFORCING MAY BE SPLICED WHERE APPROVED BY THE ENGINEER. WHERE SPLICE LENGTHS ARE NOT SPECIFIED, USE 48 BAR DIAMETERS IN MASONRY AND 48

# CONCRETE AND REINFORCING (cont'd)

- CIØ PROVIDE CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS, UNLESS OTHERWISE NOTED BEAMS (OVER STIRRUPS)
  - 1-1/2" (#5 BARS AND SMALLER) 2" (\*6 THROUGH \*18) 1-1/2" (#5 BARS AND SMALLER)

2" (\*6 THROUGH \*18)

- COLUMNS (OVER TIES) FOOTINGS SLABS
- 1-1/2" FROM TOP AT CHANGES IN DIRECTION OF CONCRETE WALLS AND BEAMS, PROVIDE CORNER BARS OF SAME
- SIZE AND SPACING AS HORIZONTAL STEEL CI2 PROVIDE STANDARD HOOKS FOR ALL TOP REINFORCING BARS AT DISCONTINUOUS ENDS. HOOKS
- MAY BE TILTED FROM VERTICAL TO OBTAIN PROPER CONCRETE COVER. CI3 GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 6000 PSI IN SEVEN DAYS. VIBROPRUF #11, BY LAMBERT CORPORATION, OR ACCEPTED SUBSTITUTE.
- CIA ALL FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED ACCORDING TO ACI 347, RECOMMENDED STANDARD PRACTICE FOR CONCRETE FORMWORK.
- CI5 RESPONSIBILITY: THE DESIGN, CONSTRUCTION, AND SAFETY OF ALL FORMWORK SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.
- CIG ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED WHERE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS.
- CII THE CONTRACTOR SHALL EMPLOY A TESTING LABORATORY TO PREPARE TEST CYLINDERS REPRESENTING CONCRETE POURED EVERY DAY, ONE SET PER DAY OR ONE SET MINIMUM FOR EACH 50 CUBIC YARDS POURED. THE TESTING LABORATORY TECHNICIAN SHALL BE PRESENT AT THE BEGINNING OF EACH POUR. LABORATORY REPORT SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER SHOWING STRENGTH OF CONCRETE AT 1 AND 28 DAYS.

### CONCRETE MASONRY UNITS NOTES

- MI MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402-2016) AND "SPECIFICATIONS FOR MAGONRY STRUCTURES (TMS 602-2016)." REFERRED TO HEREAFTER AS (TMS).
- THE COMPRESSIVE STRENGTH OF MASONRY (f'm) SHALL BE AT LEAST 2000 psi. THIS SHALL BE M2 DETERMINED BEFORE CONSTRUCTION.
- USE THE UNIT STRENGTH METHOD TO DETERMINE I'M. DETERMINE I'M BASED ON STRENGTH OF UNIT AND TYPE OF MORTAR SPECIFIED. UNITS SHALL CONFORM TO ASTM COO, TYPE II, NORMAL WT. AND SHALL BE TESTED IN ACCORDANCE WITH ASTM CI40 THICKNESS OF BED JOINTS SHALL NOT
- EXCEED 5/8 IN. LAID IN RUNNING BOND ONLY. M4 THE G.C. SHAL SUBMIT MIX DESIGNS AND TEST RESULTS FOR MORTAR AND GROUT BEFORE CONSTRUCTION BEGINS.
- MORTAR SHALL COMPLY WITH ASTM C270, TYPE M FOR RETAINING WALLS AND WALLS BELOW GRADE, TYPE & FOR TYPICAL WALLS. (COMPRESSIVE STRENGTH = 2500 PSI AND 1800 PSI RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH
- HAND MIXING MORTAR IS NOT ALLOWED. MI PIGMENTS WILL NOT BE ALLOWED IN MORTAR MIX.
- GROUT AND MORTAR SHALL BE FIELD TESTED AS DESCRIBED IN SECTION 3.1 (SMS) AND M8
- SPECIFICATIONS. M9 GROUT FOR FILLED CELLS SHALL CONFORM TO ASTM C476, LATEST REVISION, AND SHALL HAVE A SLUMP OF BETWEEN 8" AND 10". PUMP 4'-0" MAXIMUM GROUT LIFTS WITH 60 MIN. DELAY BETWEEN LIFTS. GROUT COMPRESSIVE STRENGTH SHALL BE 3000 PSI AT 28 DAYS. ALL MASONRY BELOW SLAB OR GRADE SHALL BE SOLIDLY GROUTED. GROUT SHALL BE SAMPLED
- 4 TESTED ACCORDING TO ASTM C 1019 AT A FREQUENCY OF ONCE PER LIFT. MIO SAMPLING AND TESTING WILL BE IN ACCORDANCE W/ SECTION 1.6 - TABLE 4 - LEVEL II QUALITY ASSURANCE (SMS).
- THE G.C. WILL PROVIDE CERTIFICATION FOR REINFORCING STEEL, JOINT REINFOREMENT, ANCHOR BOLTS, TIES, ANCHORS, METAL ACCESSORIES, AND CMU UNITS TO BE USED.
- MI2 REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.
- MI3 MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED: PLATE AND BENT BAR ANCHORS: ASTM A512 GRADE 50.
  - SHEET METAL ANCHORS AND TIES: ASTM A366/A366M WIRE MESH TIES: ASTM A 185 OR ASTM A 491. WIRE TIES AND ANCHORS: ASTM A 82, 4 ASTM A161, TYPE 304.
- REINFORCE JOINTS WITH LADDER-TYPE REINFORCEMENT CONFORMING TO ASTM A 951 AT 16" O.C. MEASURED VERTICALLY. LAP ALL JOINT REINFORCEMENT 6" MIN.
- MIS REINFORCE MASONRY OPENINGS GREATER THAN I'-O" WIDE, WITH HRIZ JOINT REINF PLACED IN (2) HORIZ JOINTS APPROXIMATELY 8" APART, IMMEDIATELY ABOVE THE LINTEL AND IMMEDIATELY BELOW THE SILL. EXTEND REINFORCING A MINIMUM OF 2'-O" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS. SEE PLAN FOR ADDITIONAL REQUIREMENTS
- MIG EXTEND ALL VERTICAL WALL REINFORCEMENT TO WITHIN 2" OF TOP OF WALL OR BEAM UN.O. TERMINATE REINFORCING WITH STANDARD ACI 90 DEGREE HOOK IF ROOF JOISTS AND/OR TRUSSES BEAR ON TOP OF WALL AND THERE IS NO PARAPET. IF PARAPET EXISTS HOOK IS NOT REQUIRED.
- MIJ MASONRY CONSTRUCTION JOINTS SHALL BE LOCATED AT ALL RETURNS AND SPACED NO GREATER THAN 24'-0". JOINTS SHALL ALSO BE PLACED AT A MINIMUM OF 2'-8" FROM OPENINGS. ALSO SEE DETAILS.
- MI8 JOINT FILLERS SHALL BE A PREMOLDED 3/8" JOINT FILLER.
- MIS OPENINGS SHALL HAVE A MINIMUM OF ONE BLOCK CELL AT EACH JAMB GROUTED AND REINFORCED ALSO SEE DETAILS.
- M20 PROVIDE PRECAST CONCRETE LINTEL @ ALL OPENINGS U.N.O. PROVIDE 8" BEARING EACH END, MIN. REINFORCE W/I-#5, EXTENDED 2'-0" EACH END. REFER TO DETAILS.
- M21 AT FILLED CELLS, LAY UNITS WITH FULL BED JOINTS AROUND CELLS. USE PLAIN END TWO CELLED UNIT.
- M22 UNLESS NOTED OTHERWISE BOND BEAMS SHALL BE PLACED PER NOTES ON ROOF PLAN.
- M23 USE CORED HOLES W/ STEEL SLEEVES WHEN OPENINGS ARE REQUIRED FOR DRAIN PIPES. AVOID REINF. CELLS.
- M24 WHERE CONCRETE BEAMS ARE INSTALLED IN CONCRETE BLOCK WALL, SUPPORT CONCRETE WITH 6" WIDE CONTINUOUS STRIPS OF POUR STOP MATERIAL (DUR-O-STOP OR EQUIV.) USE OF ROOFING FELT STRIPS OR ALUMINUM WILL NOT BE PERMITTED.
- M25 IF TEMPRATURE FALLS BELOW 40 DEG F. OR EXCEEDS 100 DEG. F SPECIAL CONSTRUCTION MEASURES SHALL BE TAKEN AS PER FBC 2104.3 AND 2104.4.
- M26 ALL MASONRY WALLS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF LATERAL SUPPORT BY CONNECTIONS AT FLOORS OR ROOF FRAMING LEVELS.
- M21 GROUT PLACEMENT STOPPED FOR ONE HOUR OR MORE SHOULD BE STOPPED 11/2" BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A SHEAR KEY FOR SUBSEQUENT GROUTING. M28 TYPICAL VERTICAL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.

## TIMBER TRUSSES

- TTI A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED), SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- TT2 SHOP DRAWING SUBMITTALS, INCLUDING, BUT NOT LIMITED TO, PLANS, DETAILS AND CALCULATIONS SHALL BE SUBMITTED TO ARCHITECT FOR REVIEW PRIOR TO FABRICATION. CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION. "SOUTHARD ENGINEERING, INC." WILL REQUIRE THAT THE ENGINEERED DRAWINGS FOR THE WOOD TRUSSES BE REVIEWED FOR COMPATIBILITY WITH THE DESIGN INTENT OF THE STRUCTURE PRIOR TO FABRICATION. ANY AND ALL COSTS ASSOCIATED WITH FABRICATING TRUSSES FROM SUBMITTALS NOT BEARING OUR SHOP DRAWING STAMP AND APPROVAL WILL BE THE SOLE RESPONSIBILITY OF THE G.C.
- TT3 ALL TRUSSES SHALL BE DESIGNED BY A SPECIALTY ENGINEER WITH A MINIUM OF 5 YEARS EXPERIENCE IN SIMILAR TYPE STRUCTURES.
- TT4 ALL TRUSSES SHALL BE TEMPORARILY AND PERMANENTLY BRACED AS REQUIRED BY TRUSS MANUFACTURER.
- TT5 USE SOUTHERN YELLOW PINE FOR ALL TRUSS MEMBERS.
- TTG THE ROOF PLAN SHOWN HEREIN IS A GRAPHICAL REPRESENTATION ONLY. REFER TO TRUSS SUBMITTALS FOR ACTUAL LAYOUT, TRUSS PROFILES AND HOLD-DOWN REQUIREMENTS AT ALL TRUSS SUPPORTS.
- TT8 TRUSSES SHALL BE SHOP FABRICATED, INCLUDING ANY FIELD SPLICE CONNECTION COMPONENTS AND SHIPPED TO SITE IN MAXIMUM LENGTHS AND HEIGHTS. FIELD FABRICATION OF TRUSSES WILL NOT BE PERMITTED.
- TT9 PROVIDE MOISTURE BARRIER BETWEEN TRUSS BOTTOM CHORDS AND CMU WALL.
- TTIO REVIEW ARCHITECTURAL REFLECTED CEILING PLANS AND SECTIONS FOR SPECIAL CEILING CONDITIONS. INCLUDING CEILING SLOPES, TROFFERS, COFFERS, TRAYS, STEPS AND OTHER SPECIAL FEATURES.

# SYMBOLS AND ABBREV.

	ALTERNATE/ALTERNATIVE AMERICAN CONCRETE INSTITUTE ABOVE FINISHED FLOOR AMERICAN IRON AND STEEL INSTITUTE AMERICAN IRON AND STEEL INSTITUTE AMERICAN IRON AND STEEL INSTITUTE AMERICAN WELDING SOCIETY ANCHOR BOLTS ARCHITECTURE/ARCHITECTURAL AMERICAN WELDING SOCIETY BOND BEAM BOTTOM FLANGE BRACE BASE PLATE/BEARING PLATE BEARING BEAM BLOCK BOTTOM OF BUILDING CANTILEYER CENTERLINE CLEAR/CLEARANCE COLUMN CONCRETE COLUMN CONCRETE BEAM CONCRETE BEAM CONCRETE MASONRY UNIT CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION DETAIL DEFORMED BAR ANCHOR DRY FILM THICKNESS DIAMETER DIMENSION DISTANCE DOWN DRAWING EACH EACH END EACH END EACH END EACH END EACH END EACH END EACH SIDE EXPANSION DISTANCE DIGE OF SLAB EQUAL EACH SIDE EACH WAY EXISTING EXPANSION DISTING EXPANSION DISTING	ΥΫ́ Ϋ́́Ϋ́Ϋ́Ϋ́ΎΎΎΎΎΣΣΣΣΣΣΣΣΣΣΣΣ
EX FL FL FL FL FL FL G G G G G G G G G G G G G	FLORIDA BUILDING CODE FULL LENGTH WELD. WELD ENTIRE DIST. FIELD VERIFY FINISHED FLOOR FLOOR DRAIN FOOTING GAGE/GAUGE GALVANIZED GENERAL CONTRACTOR GLUE LAMINATED HEADED ANCHOR STUD HOLLOW CORE HORIZONTAL	
HP 66 HT ID IF IT J5T	HIGH POINT HOLLOW STRUCTURAL SECTION HEIGHT INSIDE DIAMETER INSIDE FACE INTERIOR JOINT JOIST	

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		DRAWN BY: JMA / KC	DATE: 02 / 20 / 25	PROJECT NO: 24-845	1 2 2
	KNOCK OUT LENGTH LONG LEG HORIZONTAL LONG LEG VERTICAL LONG ITUDINAL MANUFACTURE/MANUFACTURER MAJONRY BEAM MATERIAL MAXIMUM MECHANICAL MEZZANINNE MINIMUM MISCELLANEOUS MAJONRY OPENING METAL NOT IN CONTRACT NOMINAL NOT IN CONTRACT NOMINAL NOT TO SCALE NORMAL WEIGH TOPPING ON CENTER OPPOSITE POWER ACTUATED FASTENER PLATE PLYWOOD POUNDS PER SQUARE FOOT POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH PRECAST CONCRETE PRE - ENGINEERED PREFABRICATED PREJECTION PRESSURE TREATED PRESSURE TREATED PANEL WIDTH REFERENCE REINFORCING REINFORCING SCHEDULE SIMILAR SPACE/SPACES SPECIFICATIONS SPRUCE PINE FUR SQUARE STUD ANCHOR STAINLESS STEEL STAINLASS STEEL STAINLASS STEEL STRUCTURAL STOP ENTON SOUTHERN YELLOW PINE THICK THREAD/THREADED TIE BEAM TOP AND BOTTOM	PAUL STRESING ASSOCIATES, INC.	14617 MAIN STREET       ALACHUA, FLORIDA 32615       E-MAIL: psa@paulstresingassociates.com	IELEPHONE: (386) 462-6407 REGISTRATION NO. AR0013985 - CA NO. AA0003377	
	TONGUE AND GROOVE TOP OF CONCRETE TOP OF STEEL TRANSVERSE TYPICAL TUBE STEEL TOP OF UNLESS NOTED OTHERWISE VERTICAL VOLUME WALL FOOTING WATERPROOF WELDED WIRE FABRIC WEEP HOLE WEIGHT WIDE FLANGE WITH WITHOUT WOOD WORKING POINT				



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